Name:

Register No.: .....

# SAINTGITS COLLEGE OF ENGINEERING (AUTONOMOUS)

(AFFILIATED TO APJ ABDUL KALAM TECHNOLOGICAL UNIVERSITY, THIRUVANANTHAPURAM)

#### FIFTH SEMESTER B.TECH DEGREE EXAMINATION (S), FEBRUARY 2024 CIVIL ENGINEERING

(2020 SCHEME)

Course Code : 20CET305

Course Name: Geotechnical Engineering – II

Max. Marks : 100

## Use of attested bearing capacity charts and IS Codes are permitted.

## PART A

## (Answer all questions. Each question carries 3 marks)

- 1. Differentiate between active and passive earth pressures.
- 2. List the factors that influence the selection of type of foundations?
- 3. Explain the following.
  - a. Ultimate Bearing Capacity
  - b. Net Ultimate Bearing Capacity
  - c. Net Safe Bearing Capacity
- 4. List the different bearing capacity failure mechanisms of a shallow foundation.
- 5. Comment on the advantages and disadvantages of Plate Load Test?
- 6. What are Combined Footings? State the situations in which trapezoidal footings are recommended.
- 7. State the circumstances under which pile foundations are recommended. Give the classification of piles based on mode of load transfer.
- 8. Define negative skin friction? How does it affect the load transfer in a pile foundation?
- 9. Differentiate between disturbed and undisturbed soil samples. What is the necessity of collecting undisturbed soil samples?
- 10. Distinguish between a Boring Log and Soil Profile.

## PART B

## (Answer one full question from each module, each question carries 14 marks) MODULE I

11. Compute the depth of tension crack and safe height for a vertical cut in case of a 6m high wall retaining in a purely cohesive soil of c' = 30 kN/m<sup>2</sup> and effective unit weight of 17.5 kN/m<sup>3</sup>. Assume the back face of the wall to be smooth and vertical. Also compute:

a. Active earth pressure and point of application before the

**Duration: 3 Hours** 

(14)

formation of tension crack.

b. Active earth pressure after the formation of tension crack.

## OR

- 12. a) A 7 m high retaining wall with vertical face supports a backfill of unit weight 17.5 kN/m<sup>3</sup>, cohesion of 11 kN/m<sup>2</sup> and angle of internal friction of 28°. There is a uniform surcharge of 25 kN/m<sup>2</sup> (8) on the backfill and the water table is well below the level and has no influence. Determine Rankine's passive earth pressure.
  - b) With neat sketches, explain any two types of shallow foundations and their specific uses. (6)

### **MODULE II**

- 13. a) A square footing of 1.5 m side carries a uniaxial load of 800 kN. The depth of foundation is 2 m and cohesion of the soil is 18 kN/m<sup>2</sup>, angle of internal friction 25° and unit weight 17 kN/m<sup>3</sup>. Assume general shear failure and the factor of safety as 3. Compute allowable bearing pressure of the foundation, if: (10)
  - i. Water table is at a great depth and
  - ii. Water table is at the ground surface (G = 2.6, e = 0.7 and  $\Upsilon_w$  = 10 kN/m<sup>3</sup>, S = 1)
  - b) List the assumptions in Terzaghi's bearing capacity theory.

#### (4)

(6)

(7)

## OR

- 14. a) A circular footing carries a uniaxial load of 600 kN at a depth of 1.5 m in medium dense sand. If the angle of internal friction of soil is 20° and unit weight is 18 kN/m<sup>3</sup>, using Terzaghi's bearing capacity theory, find the allowable bearing pressure for a factor of safety 3. What will be the mode of bearing capacity failure and why?
  - b) Discuss the factors influencing the bearing capacity of soils.

## **MODULE III**

- 15. a) Explain Plate Load Test with neat sketches.
  - b) Compute the expected settlement of a 1.6 m square footing in a granular soil, if the 30 cm plate settled by 16 mm for a load (7) intensity of  $180 \text{ kN/m}^2$  during the Plate Load Test on the same soil.

## OR

- 16. a) A trapezoidal footing is to be designed to support two square columns of 0.4 m and 0.5 m sides respectively. Center to center spacing between the columns are 5 m and safe bearing capacity of the soil is 380 kN/m<sup>2</sup>. The larger column carries 4500 kN and smaller ones carries 2800 kN. Design a suitable size of the footing so that it does not extend beyond the face of the columns.
  - b) Explain the design principles of a raft foundation.

(4)

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(7)

## **MODULE IV**

- 17. a) Discuss the estimation of bearing capacity of a single pile in clay and sand with the help of figures. (7)
  - b) A square pile group of 9 piles passes through a strata of depth 5 m which was filled recently. The unconfined compressive strength of the strata consisting of cohesive soil is 55 kN/m<sup>2</sup> and unit weight is 16 kN/m<sup>3</sup>. Compute the negative skin friction of the pile group if α = 0.4, diameter of the pile is 350 mm and center to center spacing between piles is 1 m.

### OR

- 18. a) Discuss Pile load test in detail and explain the computation of allowable load from the test. (7)
  - b) Design a friction pile group to carry a load of 3500 kN in a clay stratum of 25 m depth underlain by a hard stratum. The unconfined compressive strength of the soil is 82 kN/m<sup>2</sup> and consider a safety factor of 2.5.

### **MODULE V**

- 19. a) An SPT was conducted in a sand deposit of 15 m where the water table is at a depth of 6m from ground surface. The unit weight of sand above the water table is 18.5 kN/m<sup>3</sup> and 19.2 kN/m<sup>3</sup> below the water table. If the N value is 34, find the corrected N value.
  - b) Discuss any three methods of sub-surface exploration with sketches in detail. (8)

## OR

- 20. a) Discuss the two geophysical methods of soil exploration. What are the advantages and disadvantages of both? (7)
  - b) Compute the inside clearance, outside clearance, and area ratio of a sampler with following dimensions.
    Inside diameter of sampling tube: 70 mm
    Outer diameter of sampling tube: 74 mm
    Inside diameter of cutting edge: 68 mm and
    Outside diameter of cutting edge: 76 mm.
    Does this sampler yield undisturbed soil samples?

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