# APJ ABDUL KALAM TECHNOLOGICAL UNIVERSITY 

## FIRST SEMESTER M.TECH DEGREE EXAMINATION

## Civil Engineering

## (Geomechanics and Structures)

## 04 CE 6307 Advanced Design of Concrete Structures

Max. Marks: 60
Duration: 3 Hours

# Answer all Questions <br> Use of IS-456-2000, IS 13920:1993 and Interaction curves are permitted. Assume suitable data wherever necessary 

PART A

1. Explain effective length of a column.
2. What are yield lines? State the characteristic features of yield line.
3. What are grid floors?
4. What are the assumptions in the portal method of analysis of building frames subjected to horizontal loads?
5. What do you mean by moment redistribution?
6. Explain the curvature of a member at section with reference to figure. Draw moment (M) and curvature ( $\varnothing$ ) for a singly reinforced beam section.
7. Write a short note on the fire resistance of structural members.
8. Draw the ductile detailing of a beam-column joint.
(8x3marks=24)
PART B
9. A simply supported reinforced concrete beam of rectangular cross section 250 mm wide and 450 mm deep is used for an effective span of 4 m . The beam is reinforced with 3 nos of 20 mm diameter bars at an effective depth of 400 mm . Self-weight of the beam together with dead load on the beam is $4 \mathrm{kN} / \mathrm{m}$. Live load is $10 \mathrm{kN} / \mathrm{m}$. Using M20 grade concrete and Fe415 grade steel, compute short term deflection.

OR
10. Design a corbel to carry a girder reaction of 450 kN at a distance of 200 mm from the face of the column of size $300 \mathrm{mmx300mm}$. Assume M20 grade concrete and mild steel reinforcement. Sketch the reinforcement details also.
11. Design the interior panel of the flat slab for an office floor to suit the following data.

Size of office floor- $25 \mathrm{~m} \times 25 \mathrm{~m}$
Size of panel- $5 m \times 5 m$
Loading class- $4 \mathrm{kN} / \mathrm{m}^{2}$. Use M20 concrete and Fe 415 grade steel. Calculate the thickness of slab for the column strip and middle strip and the reinforcements.
12. Design the interior panel of a flat slab for a live load of $4000 \mathrm{~N} / \mathrm{m}^{2}$. The slab is provided with a floor finish of $1000 \mathrm{~N} / \mathrm{m}^{2}$. The panels are 6 mx 6 m . Drops shall be provided. Use M20 concrete and Fe 415 steel.
13. Two shear walls are to be provided in each direction in a 10-storey building to resist the following loadings. Floor-Floor height $=3 \mathrm{~m}$. Design the ductile shear wall to resist the forces using M25 grade concrete and TOR steel (Fe415 grade). Thickness of wall $=250 \mathrm{~mm}$, Length $=4.16 \mathrm{~m}$.

| Loading | Axial force (kN) | Moment (kNm) | Shear(kN) |
| :---: | :--- | :--- | :--- |
| 1. D.L + L.L | 1950 | 600 | 20 |
| 2. Seismic Load | 250 | 4800 | 700 |
| OR |  |  |  |

14. An RC grid floor is to be designed to cover a floor area of 12 mx 18 m . The spacing of ribs in mutually perpendicular direction is $1.5 \mathrm{~m} \mathrm{c} / \mathrm{c}$. Live load on floor is $2 \mathrm{kN} / \mathrm{m}^{2}$. Analyze the grid floor by IS-456 method. Design the suitable reinforcements (only for flexure).
15. Analyze the frame shown in figure by cantilever method.


## OR

16. Consider an intermediate frame of a multistory building. The frames are spaced at $4 \mathrm{~m} \mathrm{c} / \mathrm{c}$. Take live load as $3 \mathrm{kN} / \mathrm{m}^{2}$ and the slab thickness as 100 mm . Analyze floor level ABCD using substitute frame method. The height of columns above and below the floor $A B C D$ is 3 m . Span $A B=6 m, B C=3 m, C D=4 m$. Analyze span $A B$ and $B C$.
17. A beam $A B$ of $4 m$ span and fixed at one end and freely supported at the other end carries a udl of $30 \mathrm{kN} / \mathrm{m}$. Draw the maximum bending moment diagram as per recommendations of IS 456: 2000.
18. Calculate moment curvature for flanged beam $T$ section; $b_{f}=$ breadth of flange $=1400 \mathrm{~mm}$, $b_{w}=$ breadth of web $=300 \mathrm{~mm}$, effective depth $d=750 \mathrm{~mm}$, $D f=$ depth of flange $=150 \mathrm{~mm} . A_{s t}=$ area of steel at mid span $=1700 \mathrm{~mm}^{2}$. Use M30 and Fe 415.
19. Following are the details of an internal beam column joint of type (1) joint, subjected to reversals which are not due to earthquake. Column: $600 \mathrm{~mm} \times 600 \mathrm{~mm}$ with 8 nos 25 mm diameter bars. Column factored load is 1400 KN , Storey height=3m.Beams on either side are $400 \mathrm{~mm} \times 500 \mathrm{~mm}$ with 3 bars of 28 mm diameter $\left(1846 \mathrm{~mm}^{2}\right)$ at top and 3 bars of 25 mm diameter at bottom $\left(1473 \mathrm{~mm}^{2}\right)$.Assume fck $=25 \mathrm{~N} / \mathrm{mm} 2 \mathrm{fy}=415 \mathrm{~N} / \mathrm{mm} 2$. Design the joint with respect to strength, stability and shear.

OR
20. Explain strengthening of RC structures.

