

**APJ ABDUL KALAM TECHNOLOGICAL UNIVERSITY****Scheme for Valuation/Answer Key***Scheme of evaluation (marks in brackets) and answers of problems/key***SEVENTH SEMESTER B.TECH DEGREE EXAMINATION (S), MAY 2019****Course Code: CE401****Course Name: DESIGN OF STEEL STRUCTURES**

Max. Marks: 100

Duration: 3 Hours

PART A*Answer any two full questions, each carries 15 marks.*

Marks

- 1 a) Any three types- 3 marks (3)
 b) Strength of plate at the joint- 2 marks (12)

Strength of bolt: Shear strength-2 marks, Bearing strength-3 marks

Design strength of joint- 1 mark

Strength of solid plate-2 marks

Efficiency-2 marks

- 2 a) Any three (3)
 b) Properties of Angle ISA 125 x 95 x 8 mm, A= 1014 mm². (12)

$$\gamma_{m0} = 1.1, f_y = 250, \gamma_{m1} = 1.25$$

Design strength due to yielding of cross section by using the formula

$$T_{dg} = A_g f_y / \gamma_{m0} = 385.9 \text{ kN } \mathbf{2marks}$$

Design strength due to rupture of critical section by using the formula :

$$T_{dn} = 0.9 A_{nc} f_u / \gamma_{m1} + \beta A_{go} f_y / \gamma_{m0} = 330 \text{ kN.}$$

$$\beta = 1.4 - 0.076(w/t)(f_y/f_u)(b_s/L_c) \leq (f_u \gamma_{m0}) / (f_y \gamma_{m1})$$

$$b_s = 172 \text{ mm}, L_c = 195 \text{ mm}, \beta = 0.76 > 0.7$$

$$A_{g0} = 968 \text{ mm}^2, A_{nc} = 552 \text{ mm}^2 \quad \mathbf{4marks}$$

Design strength due to block shear (minimum of below) by using the two formulaes :

$$A_{vg} f_y / \sqrt{3} \gamma_{m0} + 0.9 A_{tn} f_u / \gamma_{m1} = 315 \text{ kN}$$

$$0.9 A_{vn} f_u / \sqrt{3} \gamma_{m1} + A_{tg} f_y / \gamma_{m1} = 288 \text{ kN}$$

$$A_{vg} = 1880 \text{ mm}^2, A_{vn} = 1264 \text{ mm}^2$$

$$A_{tg} = 320\text{mm}^2, A_{tn} = 232 \text{ mm}^2$$

4marks

Design Tensile strength of the angle = 288 kN 2marks

- 3 a) Purpose of lug angles 3marks (3)
 b) 1) **Calculation of net area** (12)

$$A_n (\text{path 11}) = (200-3 \times 22)10 = 1340\text{mm}^2$$

$$\text{Path (1221)} = (200-4 \times 22)10 + (2 \times 50^2 \times 10) / (4 \times 30) = 1536.67\text{mm}^2$$

$$\text{Path 12321} = (200-5 \times 22)10 + [4 \times 50^2 / (4 \times 30)] \times 10 = 1733.33\text{mm}^2$$

-6 marks

Mini. net area = 1340mm²
 -1 marks

Design Strength governed by yielding $T_{dg} = f_y A_g / \gamma_{m0} = (250 \times 200 \times 10) / 1.1 = 454.55\text{kN}$ -2 marks

$T_{dn} = 0.9 f_u A_n / \gamma_{m1} = 0.9 \times 410 \times 1340 / 1.25 = 395.57\text{kN}$
 -2 marks

So Design tensile strength = minimum of T_{dg} and $T_{dn} = 395.57\text{kN}$
 -1 marks

PART B

Answer any two full questions, each carries 15 marks.

- 4 a) Calculation of area of cross section required - 2 marks (15)

Calculation of design strength of section chose and showing that it is greater than 1100kN-2 marks

Two numbers of channel sections of appropriate dimensions should be chosen

Calculation of spacing of channels using the equation $2I_z = 2\{I_y + A(S/2 + C_{yy})^2\}$ - 3 marks

Fixing up of dimensions of batten (including end batten and intermediate batten) - 3marks

Calculation of compressive force coming and showing that the provided section is safe to take up the load -3marks

Provision bolted connection - 2mark

- 5 a) Explaining any three failure modes - 5marks (5)

- b) $h/b_f = 1.2$ and $t_f < 40\text{mm}$, since r_{min} is $r_{yy} = 54.1\text{mm}$, buckling class C (2) (10)

effective slenderness ratio, $\lambda = 0.624$ (1)

for buckling class C, $\alpha = 0.49$, $\phi = 0.798$ (2)

$f_{cd} = 175.44\text{N/mm}^2$ (3)

design strength of column $P_d = 1313.01 \text{ kN}$ (2)

[Full credit can be given if any student find f_{cd} using table 9(c) of IS 800]

- 6 a) 4 types- 3marks (3)
- b) Calculation of design bending moment and shear force- 2 marks , selection of beam section- 2 marks ,size of cover plates- 4 marks, check for shear- 2 marks. (12)
- Check for bearing and deflection – 2marks

PART C

Answer any two full questions, each carries 20 marks.

- 7 a) Truss configuration (2), Loads on panel points - DL on intermediate panel points =7.4 kN and on each end panel =3.7 kN (20)
- LL on intermediate panel points = 6.7 kN and on each end panel 3.35kN (3) Wind Load: Windward side : -18.8 kN and -9.4 kN and Leeward side : -17.5 kN and -8.75kN (5) . Member forces: graphically or by method of joints due to DL,LL&WL (8) , Member forces due to load combinations (2)
- 8 a) Force in any bolt due to direct load $F_1 = \frac{P}{n}$ (1) (4)
- Force in any bolt due to torque $F_2 = \frac{Per}{\Sigma r^2}$ (1)
- Resultant force acting on the critical bolt = $F = \sqrt{(F_1^2 + F_2^2 + 2F_1F_2 \cos \theta)}$ (2)
- b) Forces acting on the purlins. DL and LL calculation(2), wind load (2) (16)
- Factored bending moment and shear force (2 marks each)
- Design of section (3)
- Check for BM and SF(3)
- Check for Deflection. (2)
- 9 a) Effective span (1) (15)
- BM &SF (3)
- Design of section (2)
- Check for moment (3)
- Check for shear (2)
- Check for deflection (2)
- Check for bearing stress (2)
- b) Classification (5)
